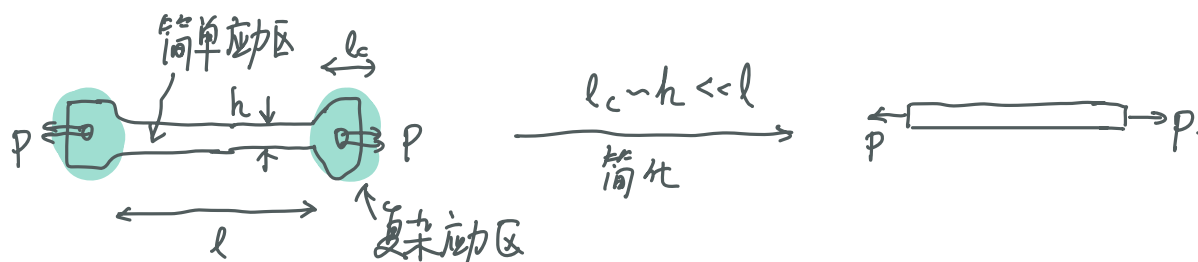


§2.1 直杆的拉伸和压缩

我们先关注直杆(条)在外力作用下产生的变形和内力. 为此, 采用以下假设(约定):

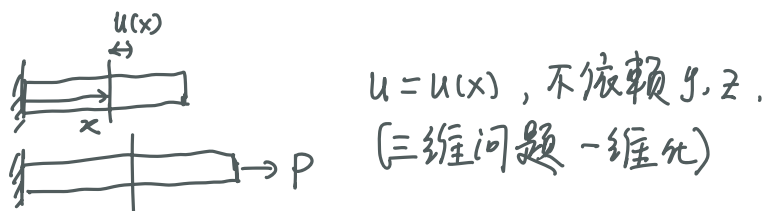
• 圣维南原理

端部作用力的形式通常较为复杂, 但该复杂性只体现在一个可忽略的范围.

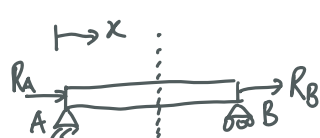


• 平截面假设

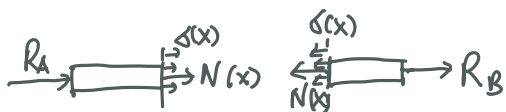
原有的横截面在变形后仍为平面, 且与轴线垂直.



• 符号约定

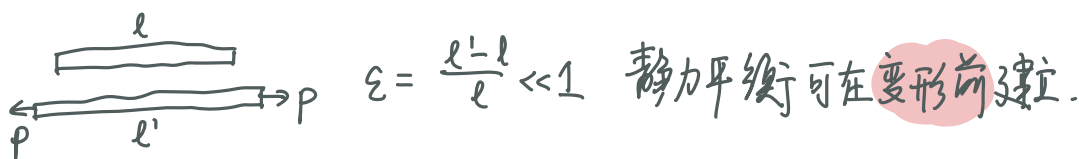


• 约束通常与坐标方向一致

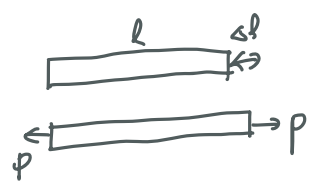


• 内力应力与截面法向方向一致, $\sigma > 0$ (拉应力)
 $\sigma < 0$ (压应力)

• 小变形假设 & Hooke's law



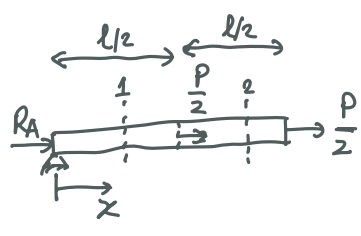
· 胡克定律



$$\epsilon = \frac{\Delta l}{l} = \frac{\sigma}{E} = \frac{P}{EA} \Rightarrow P = \underbrace{\frac{EA}{l}}_k \Delta l$$

\$k\$ - 抗拉刚度.
\$EA\$ - 截面刚度系数.

例1 杆的内力与变形



① 平衡关系

$$\sum F_x = 0 \Rightarrow R_A = -P \quad (\text{静定问题})$$

$$N = \begin{cases} P, & 0 \leq x < l/2 \\ P/2, & l/2 < x \leq l \end{cases} \quad \& \quad \sigma = \begin{cases} P/A, & 0 \leq x < l/2 \\ P/2A, & l/2 < x \leq l \end{cases}$$

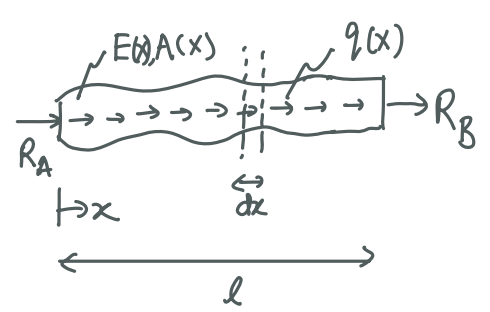
② 胡克定律

$$\epsilon = \begin{cases} \frac{P}{EA}, & 0 \leq x < l/2 \\ \frac{P}{2EA}, & l/2 < x \leq l \end{cases} \rightarrow \begin{cases} \Delta l_1 = \epsilon_1 \cdot l_1 = \frac{P}{EA} \cdot \frac{l}{2} \\ \Delta l_2 = \frac{P}{2EA} \cdot \frac{l}{2} \end{cases} \rightarrow \Delta l = \Delta l_1 + \Delta l_2 = \frac{3}{4} \frac{Pl}{EA}$$

对于这个简单的问题,我们可以直接根据“刚度”得出 $\Delta l_1 = \frac{N_1}{k_1} = \frac{P}{EA} \frac{l}{2}$, $\Delta l_2 = \frac{N_2}{k_2} = \frac{Pl}{4EA}$

但是对于更为一般形式的系统 (\$N\$ 非常数), 怎么处理? - 微元的FBD

§2.2 非均匀变形杆



① 平衡方程

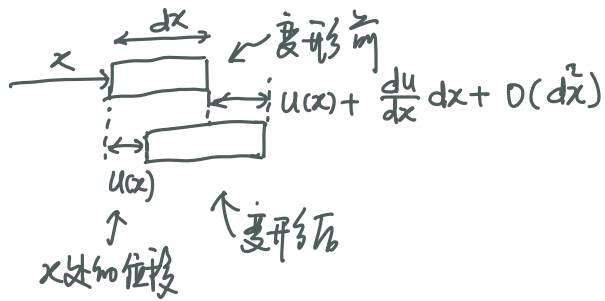
$$N(x) + q(x)dx - (N(x) + \frac{dN}{dx}dx) + O(dx^2) = 0$$

也可取左右半部得到积分方程

$$N(x) = q(x) dx + N(x) + \frac{dN}{dx} dx + O(dx^2) \rightarrow \boxed{\frac{dN}{dx} + q(x) = 0}$$

可直接积分解答: $N(x) = \underbrace{N_0}_{-R_A} - \int_0^x q(x) dx$, 以及 $\delta(x) = \frac{N(x)}{A(x)}$

② 非均匀应变



$$l = dx$$

$$l' = dx + u(x) + \frac{du}{dx} dx - u(x)$$

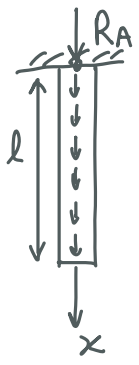
$$\boxed{\epsilon(x) = \frac{l' - l}{l} = \frac{du}{dx}}$$

③ 胡克定律

$$\epsilon(x) = \frac{N(x)}{E(x)A(x)} = \frac{du}{dx} \quad (\text{物理方程})$$

$$\text{伸长量 } u(l) - u(0) = \int_0^l \frac{du}{dx} dx = \int_0^l \frac{N(x)}{E(x)A(x)} dx$$

例 2. 自重引起的杆的应力和变形 (矿井升降机吊索 or 石油钻井机钻杆)



$$q(x) = \rho g A \quad (\text{单位长度}) \quad \rho g A l = W \quad \leftarrow \text{自重}$$

$$\rightarrow q(x) = \frac{W}{l}$$

① 平衡方程

$$\sum F_x = 0 \rightarrow R_A = -W$$

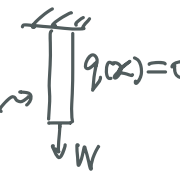
$$\frac{dN}{dx} + q(x) = 0 \rightarrow N = W - \int_0^x \frac{W}{l} dx = \frac{W}{l} (l - x)$$

$$\delta = \frac{N}{A} = \rho g (l - x)$$

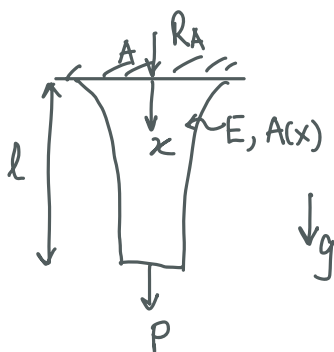
② 物理方程

$$\epsilon(x) = \frac{du}{dx} = \frac{N}{EA} = \frac{W}{EAL} (l-x)$$

$$\underbrace{u(l) - u(0)}_{\Delta l} = \int_0^l \epsilon dx = \frac{Wl}{2EA}$$

相当于 $\frac{1}{2} \rightarrow$ 

例3. 如何设计不同截面上应力值相同的杆? (使材料“强度”效益最大)



① 平衡方程

$$q(x) = \rho g A(x)$$

$$R_A = -P - \int_0^l \rho g A dx$$

$$N(x) = P + \int_0^x \rho g A dx - \int_0^x \rho g A dx = A \cdot \sigma_0 \leftarrow \text{常数}$$

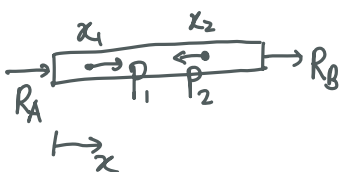
两则对x求导 $\rightarrow \frac{dA}{dx} \cdot \sigma_0 + \rho g A = 0 \rightarrow A = A(0) e^{-\frac{\rho g}{\sigma_0} x}$

此时 $R_A = -P - A(0) \sigma_0 (1 - e^{-\frac{\rho g}{\sigma_0} l})$

$$\sigma_0 = \frac{-R_A}{A(0)} = \frac{P}{A(0)} + \sigma_0 (1 - e^{-\frac{\rho g}{\sigma_0} l})$$

可以让 $\sigma_0 = \sigma_s$, P 给定, 得出 $A_c(0)$. 设计时应 $A(0) > A_c(0)$
 \uparrow 屈服应力

例4. 集中力



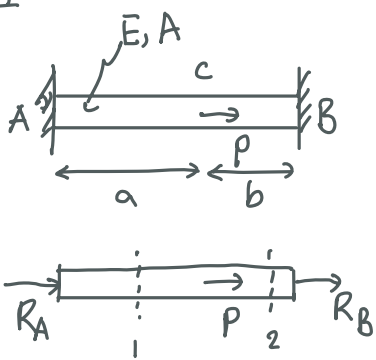
集中力也可以做为分布力 例如 $q(x) = P_1 \delta(x-x_1) - P_2 \delta(x-x_2)$

但对该问题并未带来实际上的便利. 我们将会在后续再讨论

§ 2.3 静不定问题

上面的例子中, 我仅用平衡方程便可求解支反力, 这类问题为静定问题. 但在很多问题中, 仅用平衡方程无法确定支反力 & 内力 - 静不定 (statically indeterminate) 问题. 此时, 除了平衡、物理方程外, 我们还需几何 (协调) 方程.

例 1

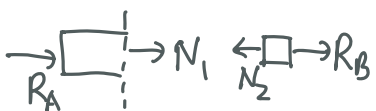


① 平衡方程

$$R_A + R_B + P = 0$$

$$N_1 = -R_A$$

$$N_2 = R_B \quad \text{4 标量, 3 方程}$$



② 物理方程

$$\Delta l_1 = \frac{N_1 a}{EA}, \quad \Delta l_2 = \frac{N_2 b}{EA} \quad \text{并未解决问题 (2 个新量)}$$

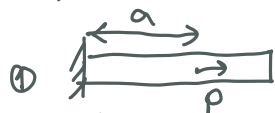
③ 几何方程

$$\Delta l_{AB} = \Delta l_1 + \Delta l_2 = -\frac{R_A a}{EA} + \frac{R_B b}{EA} = 0 \rightarrow R_A = \frac{b}{a} R_B$$

$$\rightarrow R_A = \frac{-b}{a+b} P, \quad R_B = \frac{-a}{a+b} P$$

例 2. 叠加法 (superposition) 求解上述问题

分解为两个线性问题的叠加



$$R_A^{(1)} = -P, \quad \Delta l^{(1)} = \frac{Pa}{EA}$$

+ 思想实验



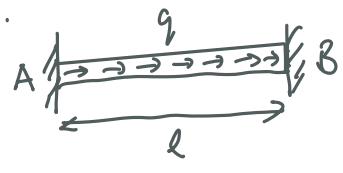
$$R_A^{(2)} = -R_B^{(2)}, \quad \Delta l^{(2)} = \frac{R_B(a+b)}{EA}$$

$$\Delta l = \Delta l^{(1)} + \Delta l^{(2)} = 0 \rightarrow R_B^{(2)} = -\frac{a}{a+b} P$$

$$R_A = R_A^{(1)} + R_A^{(2)} = -P - R_B^{(2)} = -\frac{b}{a+b} P, \quad R_B = R_B^{(1)} + R_B^{(2)} = -\frac{a}{a+b} P.$$

同样 $\Delta l_1 = \Delta l_1^{(1)} + \Delta l_1^{(2)}$, $\Delta l_2 = \Delta l_2^{(1)} + \Delta l_2^{(2)}$

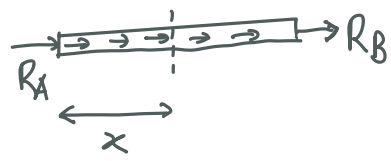
例 3.



① 平衡方程

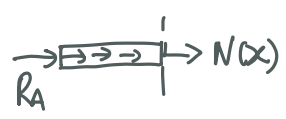
$$R_A + R_B + qL = 0$$

$$N(x) = -R_A - qx \quad (\text{也可由 } \frac{dN}{dx} + q = 0 \text{ 得出})$$



② 物理方程

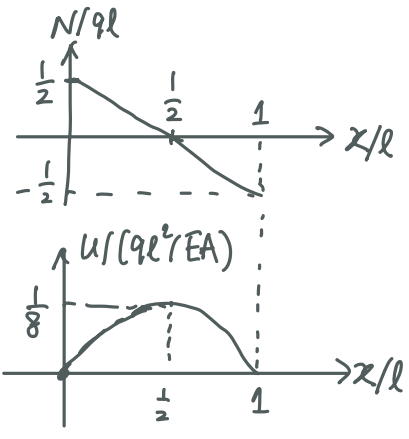
$$\frac{du}{dx} = \epsilon = \frac{N(x)}{EA} = -\frac{R_A}{EA} - \frac{q}{EA} x$$



③ 几何方程

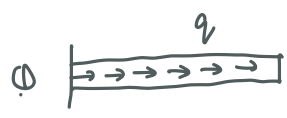
$$u(l) - u(0) = \int_0^l \left(\frac{R_A}{EA} - \frac{q}{EA} x \right) dx = 0$$

$$\rightarrow R_A = -\frac{qL}{2}, \quad R_B = -\frac{qL}{2} \quad \left(\text{Diagram of beam with } \frac{qL}{2} \text{ at ends} \right)$$



$$\rightarrow N(x) = qL \left(\frac{1}{2} - \frac{x}{l} \right), \quad u(x) = \frac{qL^2}{EA} \left(\frac{1}{2} \frac{x}{l} - \frac{1}{2} \frac{x^2}{l^2} \right)$$

也可采用叠加法: 去除左侧约束, 先施加 q ① 再施加 RB ②



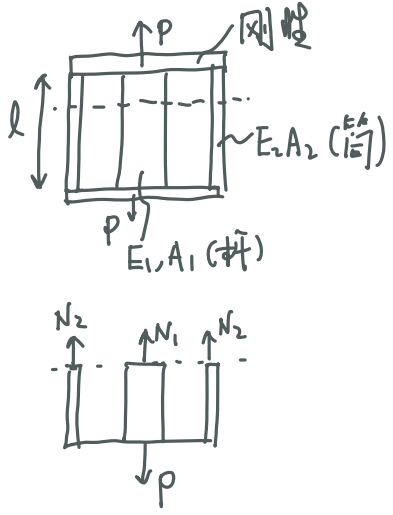
③ $\Delta l^{(1)} = \Delta l^{(3)} = \frac{qL}{2EA}$



$$\Delta l^{(2)} = -\frac{R_B l}{EA}$$

$$\Delta l = \Delta l^{(1)} + \Delta l^{(2)} = 0 \rightarrow R_B = \frac{qL}{2}$$

例 4. 组合杆



平衡方程

$$N_1 + N_2 = P$$

物理方程

$$\Delta l_1 = \frac{N_1 l}{E_1 A_1}, \quad \Delta l_2 = \frac{N_2 l}{E_2 A_2}$$

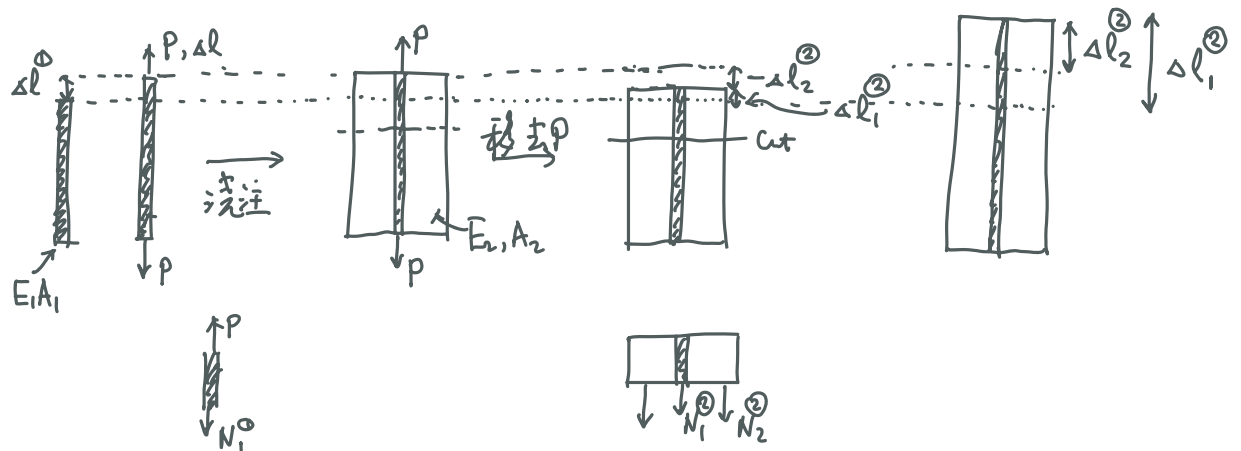
几何方程

$$\Delta l_1 = \Delta l_2$$

$$\rightarrow N_1 = \frac{E_1 A_1}{E_1 A_1 + E_2 A_2} P, \quad N_2 = \frac{E_2 A_2}{E_1 A_1 + E_2 A_2} P$$

$$\rightarrow \Delta l = \frac{P l}{E_1 A_1 + E_2 A_2} \quad \left(k = \frac{E_1 A_1}{l} + \frac{E_2 A_2}{l} = k_1 + k_2 \right)$$

例 5. 预应力混凝土



平衡方程:

$$N_1^0 = P$$

$$N_1^{(2)} + N_2^{(2)} = 0 \rightarrow N_1^{(2)} = -N_2^{(2)} = N$$

物理方程:

$$\Delta l_1^0 = \frac{P l}{E_1 A_1}$$

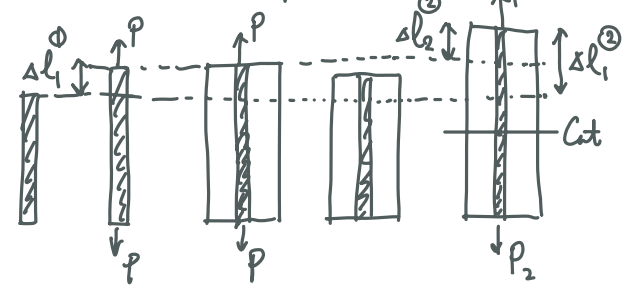
$$\Delta l_1^0 = \frac{N l}{E_1 A_1}, \quad \Delta l_2^{(2)} = \frac{-N l}{E_2 A_2}$$

几何方程:

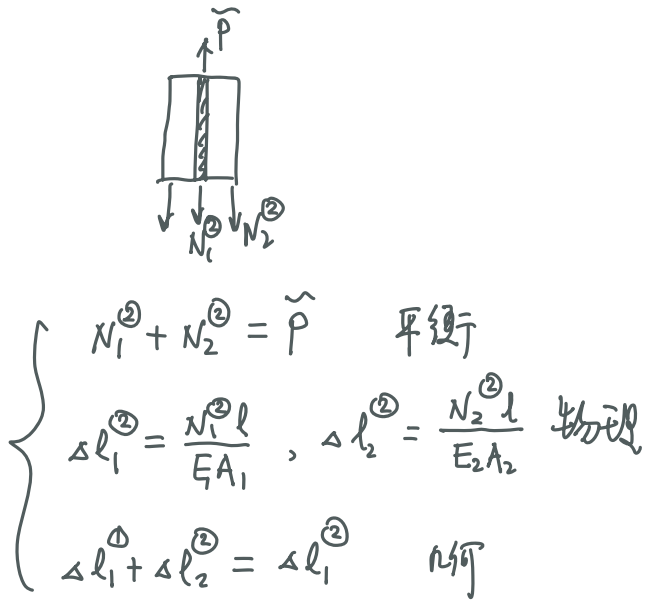
$$\Delta l_1^0 = \Delta l_1^{(2)} + (-\Delta l_2^{(2)}) \quad (\text{不依赖如何画 FBD})$$

$$\rightarrow N = \frac{E_2 A_2}{E_1 A_1 + E_2 A_2} P, \quad \sigma_1 = \frac{E_2 A_2}{(E_1 A_1 + E_2 A_2) A_1} P, \quad \sigma_2 = \frac{-E_2}{E_1 A_1 + E_2 A_2} P \quad (\text{受压})$$

再施加力 \tilde{P} ，产生多少变形？



$$\Delta l_1^0 = \frac{Pl}{EA_1}$$



$$\begin{cases} N_1^{(2)} + N_2^{(2)} = \tilde{P} & \text{平衡} \\ \Delta l_1^{(2)} = \frac{N_1^{(2)} l}{EA_1}, \Delta l_2^{(2)} = \frac{N_2^{(2)} l}{E_2 A_2} & \text{物理} \\ \Delta l_1^{(1)} + \Delta l_2^{(2)} = \Delta l_1^{(2)} & \text{几何} \end{cases}$$

$$\rightarrow N_1^{(2)} = \frac{E_2 A_2 P + E_1 A_1 \tilde{P}}{E_1 A_1 + E_2 A_2}, \quad N_2^{(2)} = \frac{E_2 A_2}{E_1 A_1 + E_2 A_2} (\tilde{P} - P) \quad \tilde{P} = P, N_1^{(2)} = P, N_2^{(2)} = 0 \checkmark$$

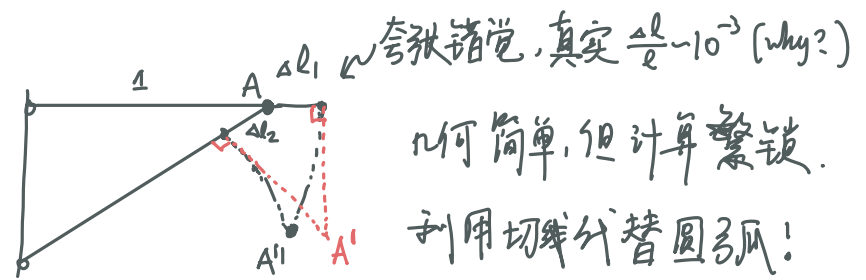
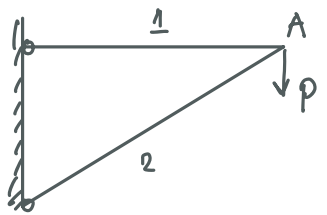
$$\begin{aligned} \text{相对变形 } \Delta l(\tilde{P}) - \Delta l(\tilde{P}=0) &= \frac{N_1^{(2)}(\tilde{P}) l}{E_1 A_1} - \frac{N_1^{(2)}(\tilde{P}=0) l}{E_1 A_1} \quad (\text{也可采用 } N_2^{(2)}) \\ &= \frac{\tilde{P} l}{E_1 A_1 + E_2 A_2} = \frac{\tilde{P}}{k_1 + k_2} \end{aligned}$$

∴ 预应力可以使得混凝土在 $\tilde{P} < P$ 内保持压缩状态，但并不会改变整体刚度！

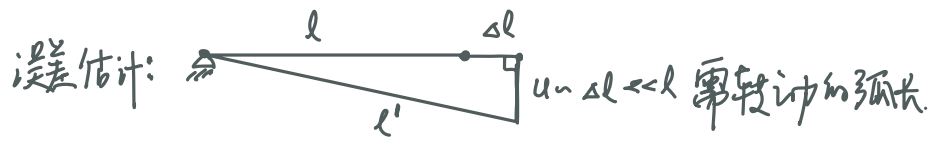
§2.4 简单桁架

桁架结构中，各杆只受轴力，且在节点处可自由转动。我们已经学习了通过 FBD 求解第 i 杆中的轴力 N_i ，根据物理方程可知其伸长量为 $\Delta l_i = \frac{N_i l_i}{E_i A_i}$ ，如何求节点

的变形？——维利奥特 (Williot) 图



夸张错觉，真实 $\frac{\Delta l}{l} \sim 10^{-3}$ (why?)
 几何简单，但计算繁锁。
 利用切线代替圆弧！

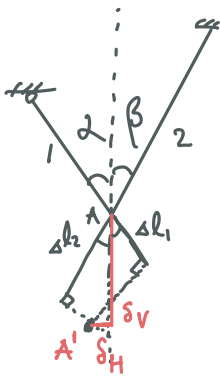


$$l' = \sqrt{(l+\Delta l)^2 + u^2} = (l+\Delta l) \sqrt{1 + \left(\frac{u}{l+\Delta l}\right)^2} = (l+\Delta l) \left[1 + \frac{1}{2} \left(\frac{u}{l+\Delta l}\right)^2 + \dots \right] \approx l + \Delta l$$

↑ 高阶量.

→ 小变形下, 切线代替圆弧的误差为 $O\left(\frac{\Delta l^2}{l^2}\right)$, 更关注 $\frac{\Delta l}{l} \sim 10^{-3}$ 量级.

例 1

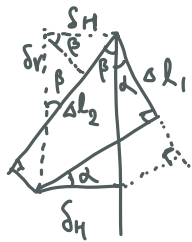


$\delta_H (\downarrow)$, $\delta_V (\leftarrow)$ 分别为水平和垂直方向位移.

几何方程:

$$\Delta l_2 = \delta_H \sin \beta + \delta_V \cos \beta$$

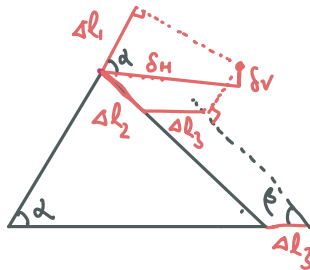
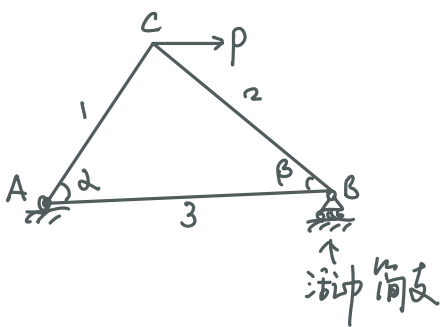
$$\Delta l_1 = \delta_V \cos \alpha - \delta_H \sin \alpha$$



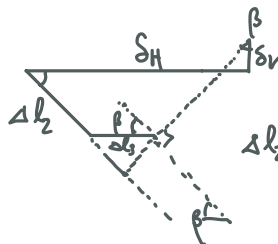
$$\rightarrow \delta_H = \frac{\Delta l_2 \cos \alpha - \Delta l_1 \cos \beta}{\sin(\alpha + \beta)}, \quad \delta_V = \frac{\Delta l_1 \sin \beta + \Delta l_2 \sin \alpha}{\sin(\alpha + \beta)}$$

Check: $l_1 = l_2, \alpha = \beta, E_1 A_1 = E_2 A_2, \Delta l_1 = \Delta l_2, \delta_H = 0$ ✓ (对称条件).

例 2.

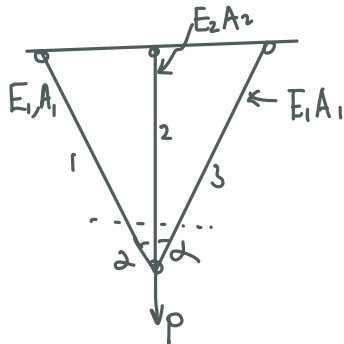


$$\Delta l_1 = \delta_H \cos \alpha + \delta_V \sin \alpha$$



$$\Delta l_2 + \Delta l_3 \cos \beta = \delta_H \cos \beta - \delta_V \sin \beta$$

例3. 静不定桁架

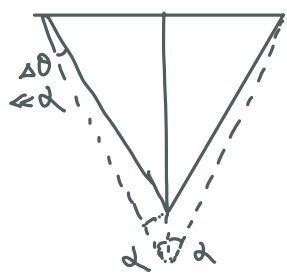


平衡方程

$$N_1 \cos \alpha + N_2 = P$$

物理方程

$$\Delta l_1 = \Delta l_3 = \frac{N_1 l}{E_1 A_1 \cos \alpha}, \quad \Delta l_2 = \frac{N_2 l}{E_2 A_2}$$



几何方程

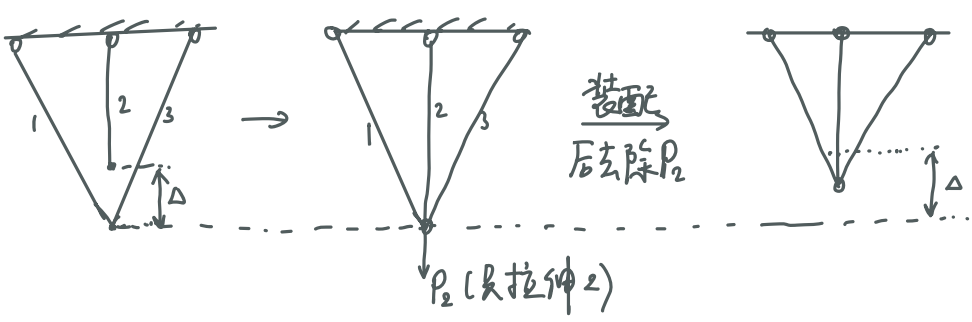
$$\Delta l_1 = \Delta l_2 \cos \alpha$$

$$\Rightarrow \frac{N_1}{E_1 A_1 \cos \alpha} = \frac{N_2}{E_2 A_2} \cos \alpha$$

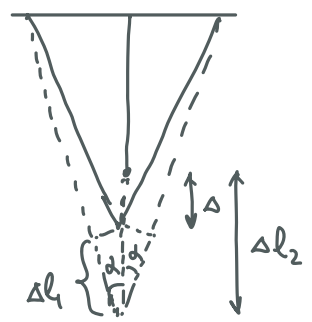
$$\rightarrow N_1 = N_3 = \frac{E_1 A_1 \cos^2 \alpha}{E_2 A_2 + 2 E_1 A_1 \cos^3 \alpha} P, \quad N_2 = \frac{E_2 A_2}{E_2 A_2 + 2 E_1 A_1 \cos^3 \alpha} P$$

$$\delta_H = 0, \quad \delta_V = PL / (E_2 A_2 + 2 E_1 A_1 \cos^3 \alpha) \quad (\text{what if } \alpha \rightarrow \frac{\pi}{2}?)$$

例4: 装配应力



几何方程

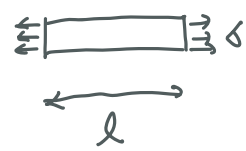


$$(\Delta l_2 - \Delta) \cos \alpha = \Delta l_1$$

思考: 再施再 P 到装配后的结构, 产生的多少位移?

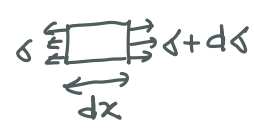
§2.5 弹性变形能.

外力所作功会以弹性变形能储存在物体内。对均匀应力的直杆，我们可以轻易得出



$$\begin{aligned}
 U &= \int_0^{Al} P d(Al) \\
 &= \int_0^\epsilon \Delta A d(\epsilon \cdot l) \xrightarrow{\text{胡克材料}} U_0 = \frac{1}{2} \frac{\sigma^2}{E} = \frac{1}{2} E \epsilon^2 = \frac{1}{2} \sigma \epsilon. \\
 &= V \int_0^\epsilon \underbrace{\sigma d\epsilon}_{U_0 \text{ 应变比能}} \\
 &\quad \uparrow \\
 &\quad Al
 \end{aligned}$$

对于非均匀应力直杆，可以考虑微元内为均匀应力：



$$\begin{aligned}
 U &= \int_V dV \int_0^\epsilon \sigma d\epsilon = A \int_0^l dx U_0(x) = \frac{1}{2} \int_0^l EA \epsilon^2 dx \\
 &= \frac{1}{2} \int_0^l \frac{N^2(x)}{EA} dx
 \end{aligned}$$

当N为常数时 $U = \frac{N^2 l}{2EA}$.

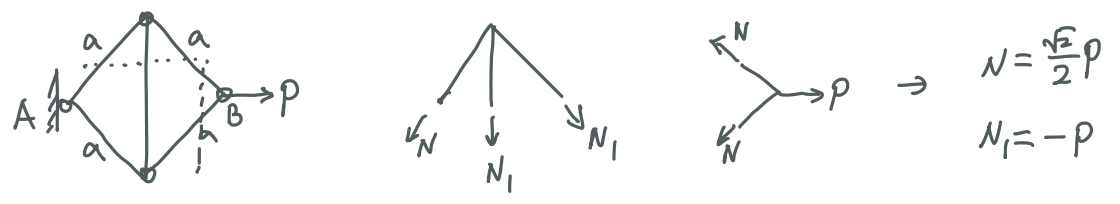
可以把弹性体看做是一个保守的力学系统，i.e., 外力功全部转化为动能和弹性变形能

$$W = T + U$$

动能在“小”加载速度下可以忽略。

对于线弹性材料杆件(系)，外力P和位移s为线性关系 $\rightarrow W = \frac{1}{2} P s = U$

例1 能量法求B处的位移



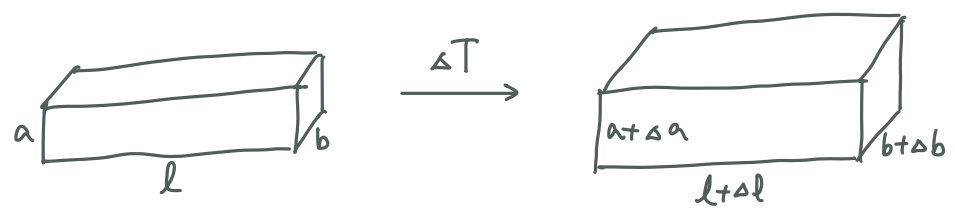
$$U = \underbrace{4 \times \frac{1}{2} \frac{N \Delta a}{EA} + 1 \times \frac{1}{2} \frac{N \sqrt{2} \Delta a}{EA}}_{(1 + \frac{\sqrt{2}}{2}) \frac{P \Delta a}{EA}} = W = \frac{1}{2} P \delta_H \rightarrow \delta_H = (2 + \sqrt{2}) \frac{P \Delta a}{EA}$$

$$\delta_V = 0 \text{ (对称性)}$$

思考: 如何处理静不定结构?

§ 2.6. 热应变

除应力外, 温度变化也会导致材料变形



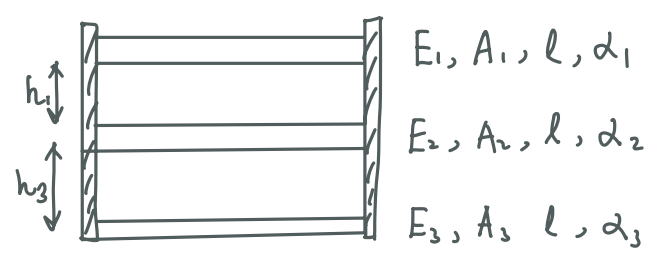
对于各项同性材料 $\epsilon_a = \frac{\Delta a}{a} = \epsilon_b = \frac{\Delta b}{b} = \epsilon_l = \frac{\Delta l}{l} = \alpha \Delta T$

↑
材料参数.

在应力和温度的共同作用下, 材料的本构关系变为:

$$\epsilon = \frac{\sigma}{E} + \alpha \Delta T$$

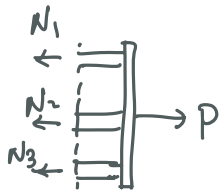
例



在拉力 P, 温度变化 ΔT 下的变形.

What if $l_2 = l + \Delta$?

① 平衡方程



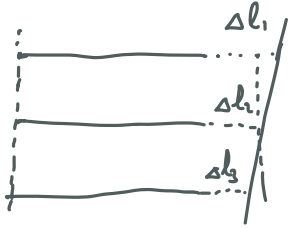
$$N_1 + N_2 + N_3 = P$$

$$N_1 h_1 + N_3 h_3 = 0$$

② 物理

$$\Delta l_1 = \frac{N_1 l}{E_1 A_1} + \alpha_1 \Delta T, \quad \Delta l_2 = \frac{N_2 l}{E_2 A_2} + \alpha_2 \Delta T, \quad \Delta l_3 = \frac{N_3 l}{E_3 A_3} + \alpha_3 \Delta T$$

③ 几何

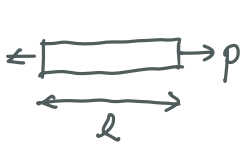


$$\frac{\Delta l_1 - \Delta l_2}{h_1} = \frac{\Delta l_2 - \Delta l_3}{h_3} \quad \text{or} \quad \Delta l_2 = \frac{h_3}{h_1 + h_3} \Delta l_1 + \frac{h_1}{h_1 + h_3} \Delta l_3$$

Check: $h_3 = 0 \rightarrow \Delta l_2 = \Delta l_3 \checkmark$.

§ 2.7 强度与刚度计算

• 刚度: 材料/结构抵抗变形的能力.



$$\Delta l = \frac{Pl}{EA} \rightarrow k = \frac{EA}{l}$$

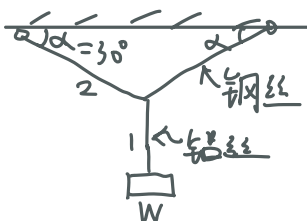
设计准则为 $u < [u]$

↑
许用位移

• 强度: 材料/结构抵抗破坏的能力.

	强度	许用应力	安全系数
脆性材料	强度极限 σ_b	$[\sigma] = \frac{\sigma_b}{n_b}$	$n_b = 2-5$
韧性材料	屈服极限 σ_s	$[\sigma] = \frac{\sigma_s}{n_s}$	$n_s = 1.5-2$

例 1.



$$N_1 = W, \quad N_2 = \frac{W}{2 \sin \alpha}$$

$d_1 = 2 \text{ mm}, [\sigma_1] = 100 \text{ MPa}$

$\sigma_1 = \frac{4W}{\pi d_1^2} < [\sigma_1] \rightarrow W < 314 \text{ N}$

$\rightarrow [W] = 188 \text{ N}$

$d_2 = 1 \text{ mm}, [\sigma_2] = 240 \text{ MPa}$

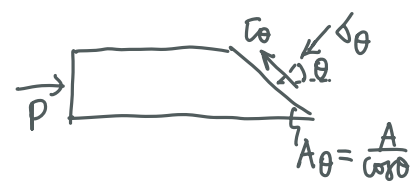
$\sigma_2 = \frac{4W}{2 \sin \alpha + d_2^2} < [\sigma_2] \rightarrow W < 188 \text{ N}$

可以提高 α , 使得 σ_2 减小, $\alpha^* = 56.4^\circ$ 时, $[W] \rightarrow 314 \text{ N}$.

例 2. 现在我们已经初步了解应力. 现考虑一块砖头, 其压缩极限强度为 σ_c , 剪切极限强度为 $\tau_c = \frac{1}{2} \sigma_c$. 如何破坏?



$\sigma \Rightarrow \sigma = \frac{P}{A}$, 没有切应力?



$\Sigma F_x = P - \sigma_\theta \cdot \cos \theta \cdot A_\theta - \tau_\theta \sin \theta A_\theta = 0$

$\rightarrow P - \sigma_\theta A - \tau_\theta \frac{\sin \theta}{\cos \theta} A = 0$

$\Sigma F_y = -\sigma_\theta \sin \theta A_\theta + \tau_\theta \cos \theta A_\theta = 0$

$\rightarrow \sigma_\theta = \frac{\tau_\theta \cos \theta}{\sin \theta}$

$\rightarrow \tau_\theta = \frac{P}{2A} \sin 2\theta, \sigma_\theta = \frac{P}{A} \cos^2 \theta$

$\sigma_{\max} = \sigma_\theta(\theta=0) = \frac{P}{A}, \tau_{\max} = \tau_\theta(\theta=\frac{\pi}{4}) = \frac{P}{2A}$

$\rightarrow P_{\max} = \sigma_c A$

$\rightarrow P_{\max} = \tau_c \cdot 2A = \frac{1}{2} \sigma_c A \therefore$ 剪切破坏

如何表征材料的剪切强度/刚度? — 第三章.

稳定性: 材料/结构保持原有平衡的能力.

